

2D ELASTICITY ANALYSIS WITH BOUNDARY ELEMENT METHOD

Supriyono

Departement of Mechanical Engineering, Faculty of Engineering,
Muhammadiyah University of Surakarta.
Email: supriyono@ums.ac.id

ABSTRACT

In this paper, a boundary element method for 2D elasticity analysis is presented. The formulations are also presented. Numerical integration is applied to solve the boundary integral equation obtained from the formulation. Quadratic isoparametric elements are used to represent the variation of a variable along an element. Several examples are presented to demonstrate the validity and the accuracy of the method.

Keywords: *elasticity-numerical integration-isoparametric element-boundary element method.*

Introduction

In general, there are three popular numerical methods used in practical problems, the Finite Difference Method (FDM), the Finite Element Method (FEM) and the Boundary Element Method (BEM). FDM and FEM are called domain methods as the discretization of the domain is required. On the other hand, the BEM (Brebbia, 1984) is known as a boundary type method. The most interesting feature of the Boundary Element Method (BEM) is that only the boundary of the model needs to be discretized, thus the dimensionality of the problem is reduced by one. It means that for two-dimensional problems, only the line-boundary of the domain needs to be discretized into elements, and for three-dimensional problems only the surface of the problem need to be discretized (see Fig. 1). Further advantages can be found in the continuous modelling of the interior and usually a coarser discretization is needed compared to Finite Element Method meshes. The BEM's applicability at present is not as wide ranging as FEM, however the method has become established as an effective alternative to

FEM in several important areas of engineering analysis.

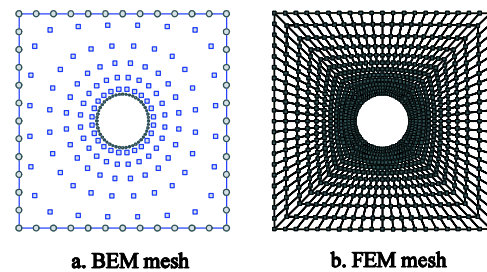


Figure 1. BEM vs FEM mesh in 2D

The BEM formulations can be divided into two different but closely related categories. The first and perhaps the most popular is the so-called direct formulation, in which the unknown functions appearing in the formulation are actual physical variables of the problem. In elasticity these unknown functions are the displacement and traction fields. The other approach is called the indirect formulation, in which unknown functions are represented by fictitious source densities. Once these source densities are found, the values

of the physical parameters can be obtained by simple integrations.

The fundamentals of the BEM can be traced back to classical mathematical formulations by Betti (1872), Somigliana (1886), Fredholm (1903), and Mikhilin (1957). The works by Fredholm and Mikhilin were for potential problems, whereas the works of Betti and Somigliana dealt with elasticity problems. The development of the formulations in the context of boundary integral equation is due to Jaswon (1963), Massonnet (1965), Hess and Smith (1967), Rizzo (1967) and Cruse (1969). Cruse was the first one who introduced three-dimensional elastostatics in boundary element method. The work of Lachat and Watson (1976) is perhaps the most significant early contribution towards BEM becoming an effective numerical technique. They developed an isoparametric formulation similar to those used in the FEM and demonstrated that BEM can be used as an effective tool for solving problems with complex configuration. Since these early contributions of the BEM, much progress has been made in many different applications. Several authors have written text books on BEM, such as Aliabadi (2001), Brebbia (1992), Banerjee(1992), Becker (Becker1992), and Wrobel (2001).

This paper presents the application of BEM to two dimensional (2D) elastostatic problems. Throughout this paper, the cartesian tensor notation is used, with the Latin indices varying from 1 to 2.

Displacement and Stress Integral Equations

Applications BEM in solid mechanics are based on the Somigliana's identities. Somigliana's identity for displacements in 2D elasticity problems states that the displacements at any points X' [$u_i(X')$] belonging to domain ($X' \in V$) to the boundary values of displacement [$u_j(x)$] and traction [$t_j(x)$] can be expressed as (Aliabadi, 2001):

$$u_i(X') = \int_S U_{ij}(X', x) t_j(x) dS - \int_S T_{ij}(X', x) u_j(x) dS \quad (1)$$

where, $U_{ij}(X', x)$ and $T_{ij}(X', x)$ are called fundamental solutions representing a displace-

ment and a traction in the j direction at point x due to a unit point force in the i direction at point X' . These fundamental solutions can be found in Aliabadi (2001).

Equation (1) is valid for any source points within domain ($X' \in V$), in order to find solutions on the boundary points, it is necessary to consider the limiting process as $X' \rightarrow x' \in S$. The limiting process can be found in many text book, for examples Aliabadi (2001), Brebbia (1992), Banerjee(1992), Becker (Becker1992), and Wrobel (2001). After limiting process, boundary displacement integral equations can be expressed as

$$C_{ij}(x') u_i(x') = \int_S U_{ij}(x', x) t_j(x) dS - \int_S T_{ij}(x', x) u_j(x) dS \quad (2)$$

where, $C_{ij}(x')$ is free term that is $C_{ij}(x') = \frac{1}{2} (\delta_{ij} + \nu_{ij}(x'))$, for smooth boundary the free term is 0.5.

The Somigliana's identity for stresses can be expressed as

$$\sigma_{ij}(X') = \int_S U_{ijk}(X', x) t_k(x) dS - \int_S T_{ijk}(X', x) u_k(x) dS \quad (3)$$

where, $U_{ijk}(X', x)$, $T_{ijk}(X', x)$ are called fundamental solutions and can be found in the same text book as mentioned above.

As equation (1), equation (3) is valid for any source points within domain ($X' \in V$), to find stresses on the boundary, two methods are available. The first commonly called as *indirect approach* relies on using recovered boundary tractions and displacements obtained from the BEM solutions using equation (2). The tangential strains are calculated by differentiation of equation (2) and then the strains are converted by Hooke's law and Cauchy's formula to have the stresses. The second method is called *direct approach*. The stresses can be obtain by limiting process of equation (3) as $X' \rightarrow x' \in S$. This method is complicated and will include hypersingular integral due to the limiting process. The first approach is the most pupolar and economical. The details of the formulations of the first approach can be found in Aliabadi (2001).

Discretization and System of Equation

In order to solve equation (2), a numerical method is implemented as analytic solution is almost impossible due to complexity of the equation. The boundary S is discretized into N_e using quadratic isoparametric elements as can be seen in Figure 2.

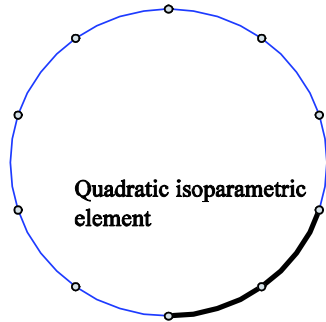


Figure 2. Discretization

In this formulation, boundary parameter x_j , the unknown boundary values of displacements u_j and tractions t_j are approximated using interpolation function, in following manner:

$$\begin{aligned} x_j &= \sum_{\alpha=1}^3 N_{\alpha}(\xi) x_j^{\alpha} \\ u_j &= \sum_{\alpha=1}^3 N_{\alpha}(\xi) u_j^{\alpha} \\ t_j &= \sum_{\alpha=1}^3 N_{\alpha}(\xi) t_j^{\alpha} \end{aligned} \quad (4)$$

The shape functions N_{α} are defined as

$$\begin{aligned} N_1 &= \frac{1}{2} \xi (\xi - 1) \\ N_2 &= (1 - \xi)(1 + \xi) \\ N_3 &= \frac{1}{2} \xi (\xi + 1) \end{aligned} \quad (5)$$

Substituting equation (4) and equation (5) into equation (2), one gets (the integrations on the boundary S):

$$\begin{aligned} \int_S T_{ij}(x', x) u_j(x) dS &= \sum_{n=1}^{N_e} \sum_{\alpha=1}^3 u_j^{\alpha} \int_{-1}^1 T_{ij}(x', x(\xi)) N_{\alpha}(\xi) J^n(\xi) d\xi \\ \int_S U_{ij}(x', x) p_j(x) dS &= \sum_{n=1}^{N_e} \sum_{\alpha=1}^3 t_j^{\alpha} \int_{-1}^1 U_{ij}(x', x(\xi)) N_{\alpha}(\xi) J^n(\xi) d\xi \end{aligned} \quad (6)$$

where, N_e is the number of elements on the boundaries S and J^n is the Jacobian transformations.

After discretization and point collocation on the boundary the equations (6) can be written in the matrix form as

$$[H]\{u\} = [G]\{t\} \quad (7)$$

where $[H]$ and $[G]$ are the well-known boundary element influence matrices. $\{u\}$, $\{t\}$, are the displacement and the traction rate vectors on the boundary.

After imposing boundary condition, equations (7) can be written as

$$[A]\{x\} = \{f\} \quad (8)$$

where, $[A]$ is the system matrix, $\{x\}$ is the unknown vector and $\{f\}$ is the vector of prescribed boundary values.

In similar way, the stress integral equations of equations (3) can be presented in matrix form as

$$[\sigma] = [G]\{t\} - [H]\{u\} \quad (9)$$

At this point, it can be seen that the stresses at internal points are calculated using equation (9) after the boundary values of displacements and tractions are found from equation (8).

Examples

In order to show the accuracy and the validity of the method presented above, examples are shown as follows:

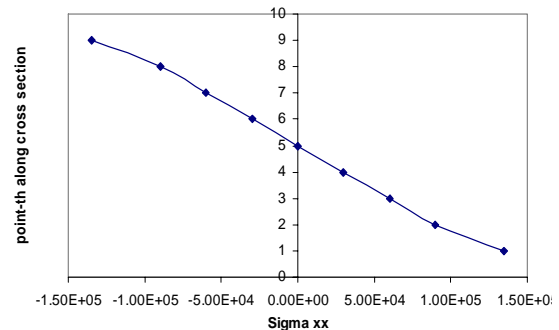


Figure 3. Distribution of normal stress along the cross section of cantilever beam

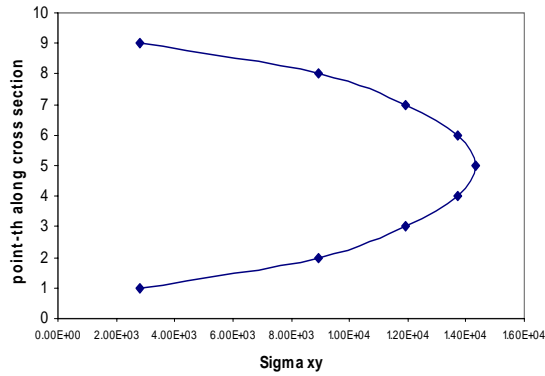


Figure 4. Distribution of shear stress along the cross section of cantilever beam

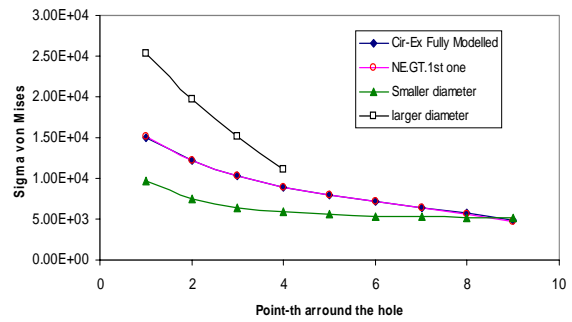


Figure 5. Stress distribution of circular excavation plate due to tension (The points was taken in the y direction along side the diameter of the hole)

Conclusion

Some points can be drawn from the presentation above:

1. The most interesting feature of BEM is reducing dimensionality of the problem by one.
2. The discretization technique of BEM is the same as FEM discretization.
3. BEM has become established as an effective alternative to FEM in several important areas of engineering analysis..

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